

# Determination of vulnerability indexes for buildings and infrastructures designed to protect the territory of Ferrara from flood - Part a -

---

Final Version

Deliverable Number D.3.3.2.

<b>Project Acronym</b>	PMO-GATE
<b>Project ID Number</b>	10046122
<b>Project Title</b>	Preventing, Managing and Overcoming natural-hazards risk to mitiGATE economic and social impact
<b>Priority Axis</b>	2: Safety and Resilience
<b>Specific objective</b>	2.2: Increase the safety of the Programme area from natural and man-made disaster
<b>Work Package Number</b>	3
<b>Work Package Title</b>	Assessment of single-Hazard exposure in coastal and urban areas
<b>Activity Number</b>	3.3
<b>Activity Title</b>	Assessment of climate-unrelated hazards exposure in urban and coastal areas (seismic action)
<b>Partner in Charge</b>	UNIFE
<b>Partners involved</b>	UNIFE
<b>Status</b>	Final
<b>Distribution</b>	Public

## Table of content

1. Introduction.....	3
2. Area description .....	4
3. Seismic hazard description .....	5
4. Geomorphology of the area .....	8
5. Subsoil model in the Cona area .....	8
5.1 Natural frequencies of the deposits .....	8
5.2 Shear waves propagation speed from SCPTU tests.....	8
5.3 Shear waves propagation speed from ESAC tests .....	8
5.4 Exposure to liquefaction.....	9
5.5 Lithostratigraphic sequence .....	9
6. Seismic microzonation of the Cona area .....	9
6.1 Computed ground motion parameters .....	10
6.2 Pseudo-acceleration response spectra.....	11
6.3 Ground motion amplification factors .....	12
Bibliography.....	13

## 1. Introduction

Cona is part of the Municipality of Ferrara and plays an especially important role in the Ferrara territory, since it hosts the main city hospital. Seismic microzonation of the Cona area has been carried out, within the broader framework of the seismic microzonation of the whole Ferrara territory, by the Municipality of Ferrara in collaboration with the Department of Engineering of the University of Ferrara, in agreement with the most recent Italian codes (O.P.C.M. 3274/2003, D.M. 14/09/2005, D.R. 102-2007, D.M. 14/01/2008, I.C.M.S. 2009). The main activities can be summarized as follows:

1. Definition of the seismic hazard for the municipality of Ferrara;
2. Geologic and geomorphologic classification of the area under study;
3. Collection of previous geotechnical and geophysical investigations;
4. Planning and implementation of 14 static penetrometric tests with seismic piezocone (SCPTU), 39 measures of environmental noise with the Horizontal to Vertical Spectral Ratio methodology (HVSr), 4 measures of the propagation speed of surface waves by means of the Extended Spatial Auto Correlation (ESAC) technique;
5. Creation of a database containing the collection of all geognostic data obtained;
6. Litostratigraphic interpretation of 643 static penetrometric tests with mechanical head (CPT), with electric head (CPTe), with piezocone (CPTU), with seismic piezocone (SCPTU) and 59 boring tests;
7. Interpretation of 99 geophysical tests (HVSr, ESAC);
8. Definition, for every penetrometric test, of the reference geotechnical profile and the principal mechanical parameters.
9. Seismic classification and evaluation of the seismic amplification factor;
10. Assessment, for every penetrometric test, of the liquefaction hazard;
11. Assessment, for every penetrometric test, of ground vulnerability to earthquake-induced settlements.
12. Estimate of the local seismic amplification for 11 locations.

## 2. Area description

The municipality of Ferrara covers a total surface of 405.16km<sup>2</sup> and is depicted in Fig. 1. Cona, depicted in Fig. 1 by a red circle, is located 10km east of the city center and its territory comprises the villages of Cocomaro di Cona, Codrea and Quartesana.

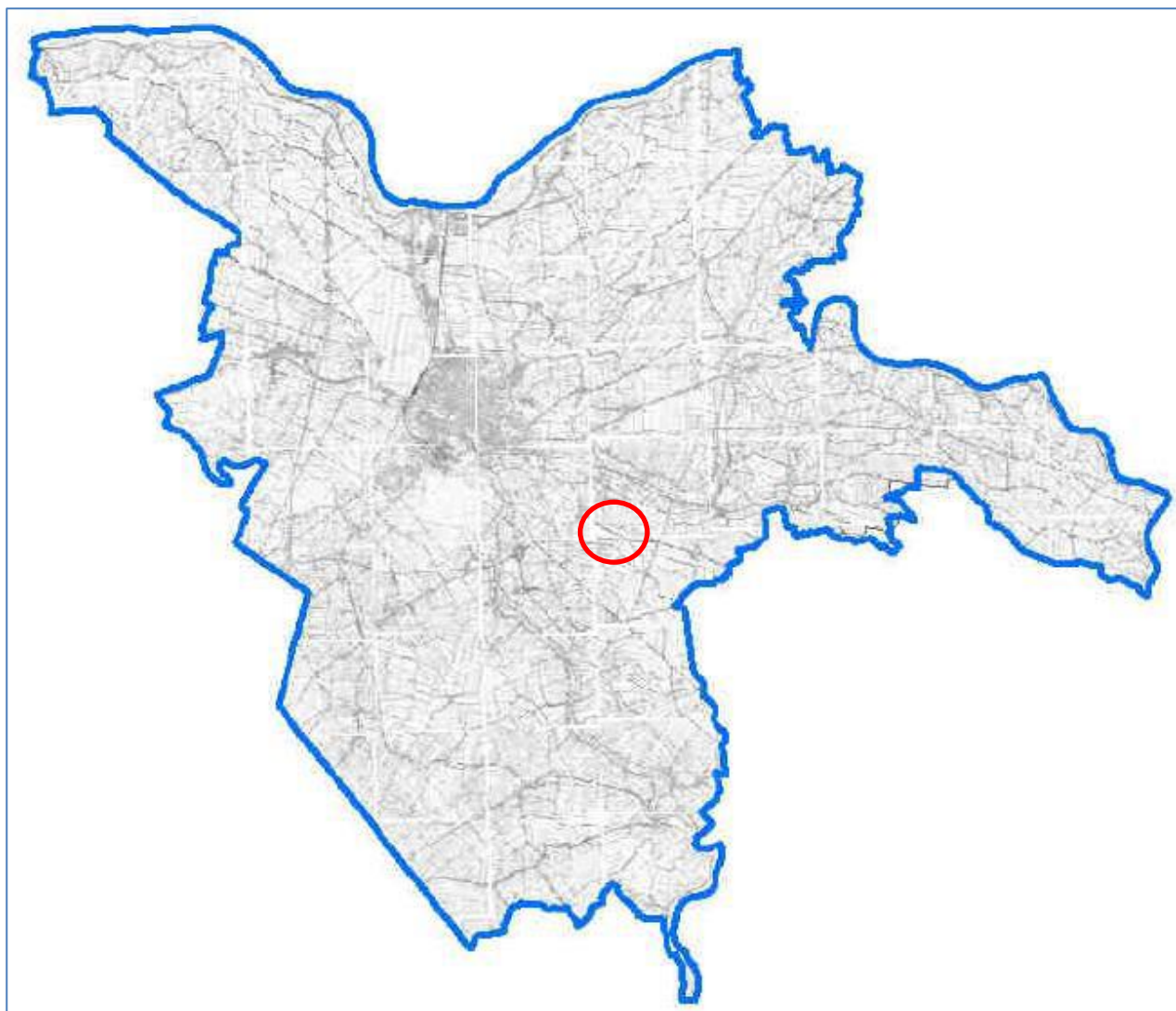


Figure 1 - The municipality of Ferrara with the Cona area in evidence

### 3. Seismic hazard description

The territory of the municipality of Ferrara falls within zone 912 of the seismic zonation ZS9, as represented in Fig. 2. The maximum expected magnitude is  $M = 6.14$ . This value has been assumed as a reference value in the analyses that follow. The territory of Ferrara was struck by destructive earthquakes in May-June 2012, with two main sequences on May 20-29 with magnitude  $M$  5.9 and 5.8, respectively. Maximum recorded horizontal accelerations were less than  $0.1g$ .

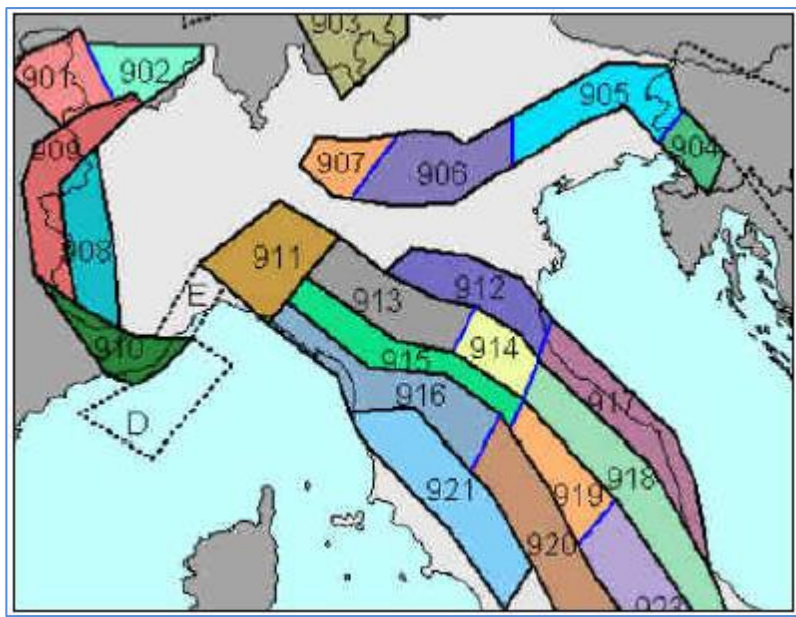


Figure 2 – Seismic zonation ZS9

The design seismic accelerations, provided by the Italian Building Code (NTC2008), are reported in Fig. 3 in terms of peak ground acceleration expected on a rigid soil ( $PGA_0$ ) on a return period  $T_r = 475$  years.





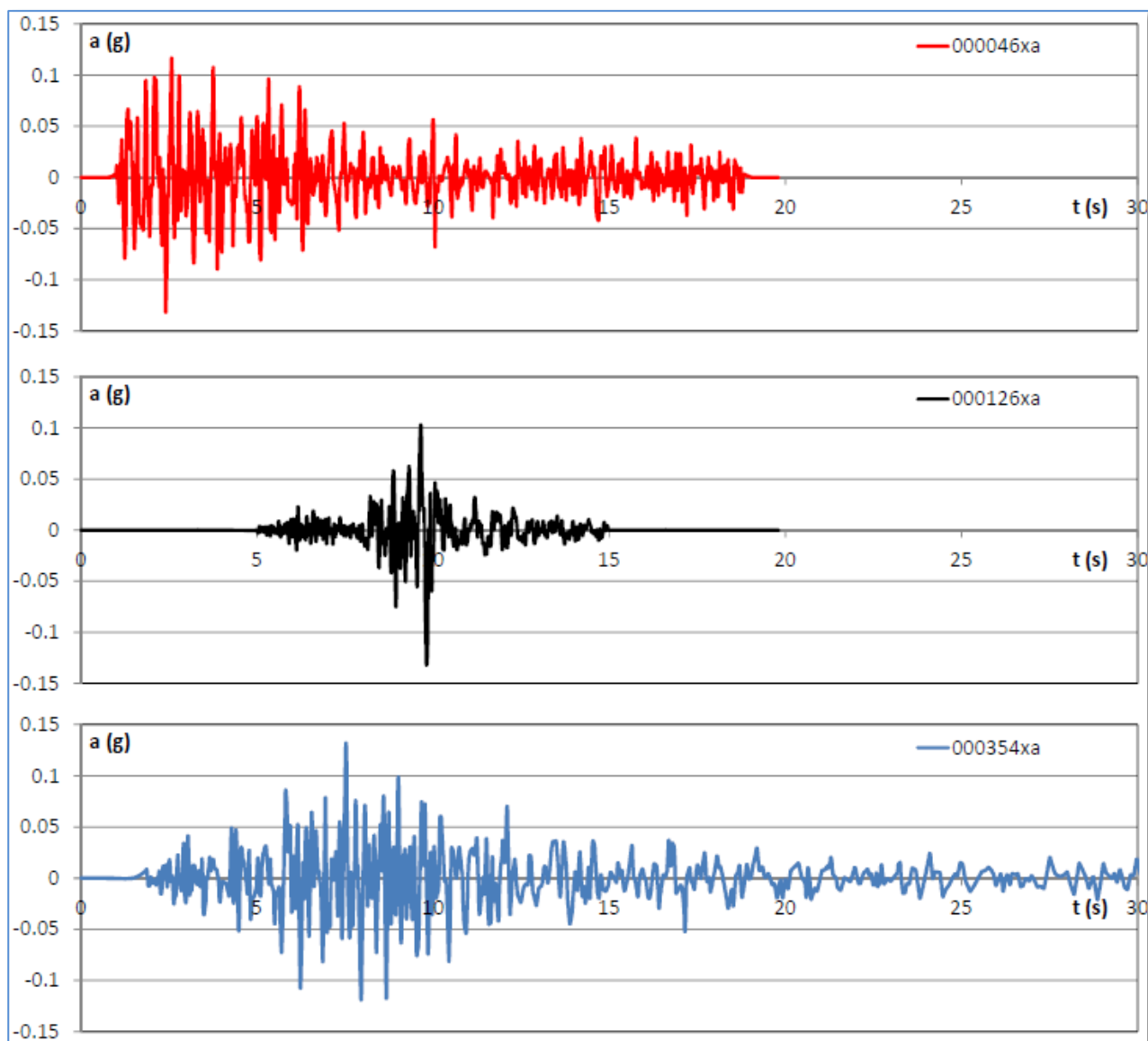


Figure 4- Reference accelerograms used for the determination of the local seismic response



codice	PGA [g]	PGV [m/s]	PGD [m]	d <sub>90</sub> [s]	I <sub>a</sub> [m/s]	SI [m]	SI <sub>05</sub> [m]	SI <sub>15</sub> [m]
000046xa_038008	0.132	0.060	0.006	13.480	0.197	0.168	0.048	0.094
000126xa_038008	0.132	0.075	0.006	3.525	0.054	0.180	0.045	0.096
000354xa_038008	0.132	0.065	0.014	15.355	0.212	0.279	0.048	0.143

Table 1 – Fundamental parameters for the reference ground motion adopted

## 4. Geomorphology of the area

The territory of the municipality of Ferrara is contained in the Po valley, a tectonic depression formed between Alps and Apennines mountain ranges. The area is located along the so-called “Pieghe Ferraresi”, arches of the orogenic belt of the Apennines made of two anticline folds oriented northward and elongated in the WNW-ESE direction. In the first 25-30m of depth in the ground, three main morphological environments can be detected:

- Paleochannel environment: is made of sands and fine grain soils.
- Interfluvial basin environment: is composed of sediments with a prevalence of fine grain soils.
- Transition environment: is made of an alternation of fine grain soils with thin layers of sands.

Groundwater level is always close to ground level, with excursions in the range of +/- 1m.

## 5. Subsoil model in the Cona area

### 5.1 Natural frequencies of the deposits

From the HVSR tests, all HVSR curves are characterized by the presence of two maxima in extra-low (0.25 Hz) and low (0.7 – 1.1 Hz) frequency, respectively.

### 5.2 Shear waves propagation speed from SCPTU tests

SCPTU measured shear waves propagation speed is 194 m/s. The Italian Building Code (NTC2008) assigns to the area soil category C.

### 5.3 Shear waves propagation speed from ESAC tests

ESAC tests for the Cona area are reported in Fig. 5.

### 5.4 Exposure to liquefaction

Differently from the rest of the municipality, the Cona area is not subjected to liquefaction hazard.

### 5.5 Lithostratigraphic sequence

The adopted lithostratigraphic sequence has been summarized in Tab. 2, in terms of lithology, soil density and shear waves propagation speed.

## 6. Seismic microzonation of the Cona area

The local seismic response analysis has been conducted by means of the ProShake analysis software.

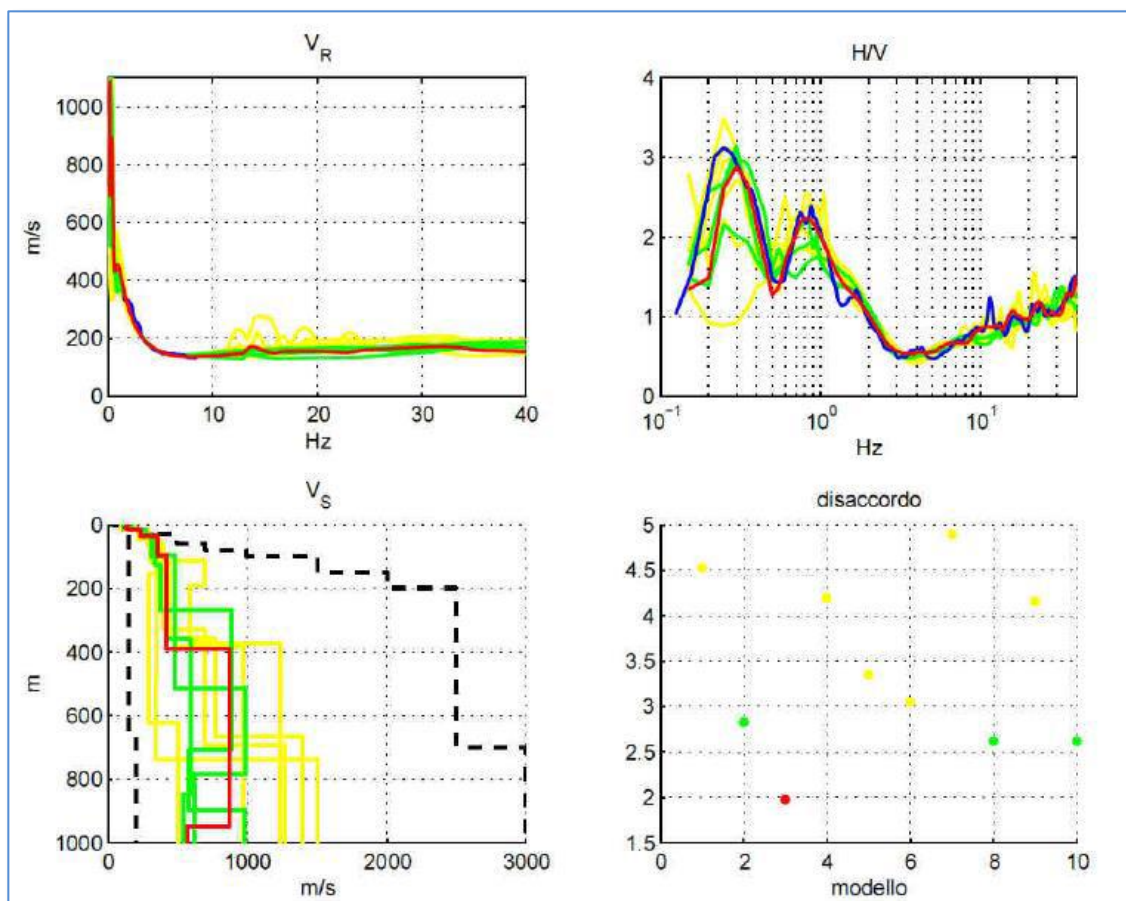


Figure 5 - ESAC tests results in the Cona area.

Table 2 - Cona lithostratigraphic sequence

Ospedale Cona				
z		LITOLOGIA	$\gamma_n$ [kN/m <sup>3</sup> ]	V <sub>s</sub> [m/s]
da [m]	a [m]			
0	6	L	18	120
6	16	A	18	150
16	30	A	18	225
30	35	A	18	270
35	50	S	18	360
50	65	S	18	440
65	85	A	18	470
85	120	S	18	665
120	140	A	18	730
>140		S	20	800

ProShake allows to simulate the one-dimensional propagation of the seismic motion in soil deposits horizontally stratified. ProShake works in the frequency domain transforming, by means of an FFT algorithm, an input time history sequence into the sum of harmonic oscillations with different frequencies. The nonlinear dissipative behavior of the soil is accounted for by means of an equivalent linear approach. The input of the program can be synthesized as:

- i) Lithostratigraphic sequence, summarized in Tab. 2.
- ii) Initial shear stiffness  $G_0$  for each soil layer, computable from the shear waves propagation speed  $V_s$  as  $G_0 = \rho_n V_s$ , where  $\rho_n$  is the soil density.
- iii) Shear stiffness decay curves  $G(\gamma)$  for each soil layer. Such curves have been determined from resonating column (RCTS) tests.
- iv) Input accelerograms, depicted in Fig. 3 and summarized in Tab 1.

### 6.1 Computed ground motion parameters

The parameters of the computed ground motions are reported in Tab. 3.

Table 3 - Computed ground motion parameters

codice	PGA	PGV	PGD	d <sub>90</sub>	I <sub>a</sub>	SI	SI <sub>05</sub>	SI <sub>15</sub>
	[g]	[m/s]	[m]	[s]	[m/s]	[m]	[m]	[m]
000046xa_038008_ampl	0.225	0.170	0.018	13.260	0.650	0.446	0.105	0.265
000126xa_038008_ampl	0.299	0.163	0.021	3.440	0.235	0.507	0.106	0.296
000354xa_038008_ampl	0.228	0.159	0.023	18.740	0.908	0.679	0.108	0.397

## 6.2 Pseudo-acceleration response spectra

The corresponding pseudo-acceleration spectra, determined from the local seismic response analysis, are reported in Fig. 6.

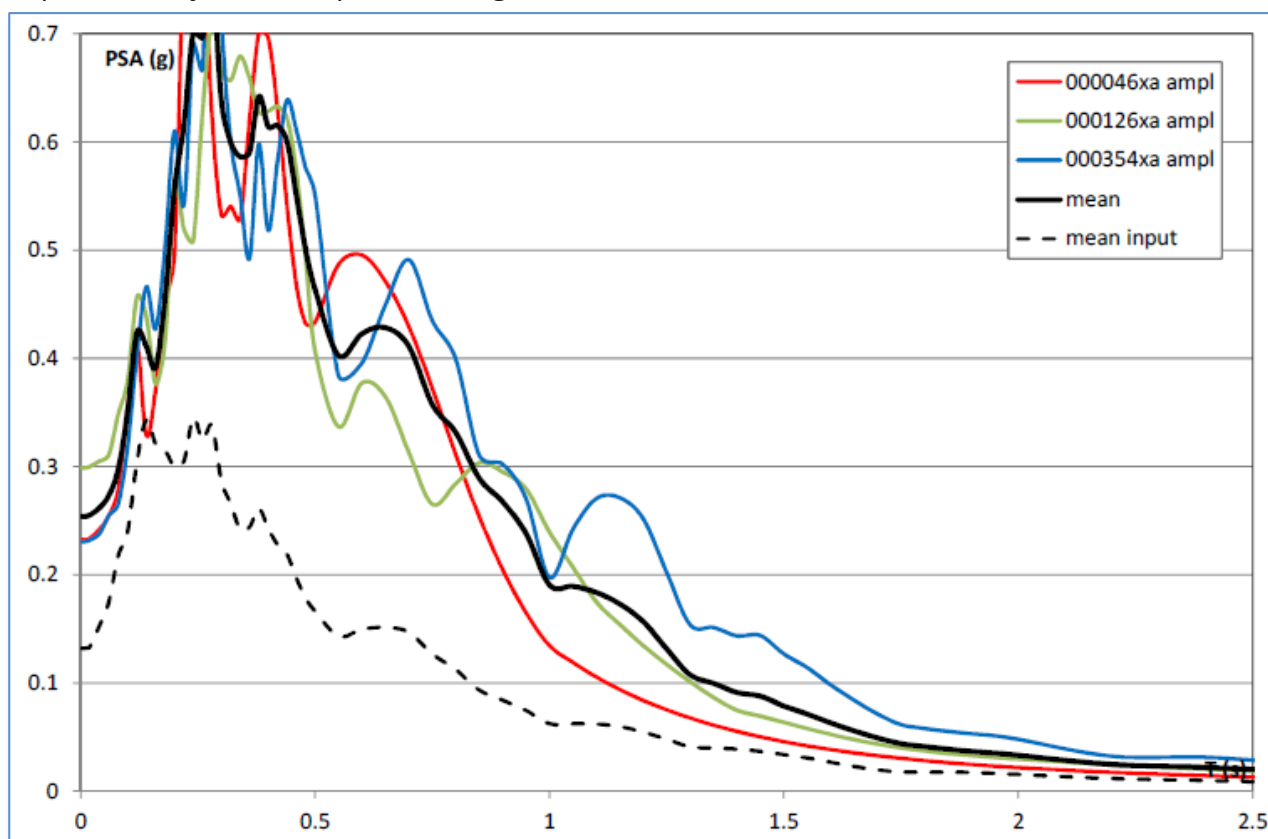


Figure 6 - Pseudo-acceleration spectra for the Cona area obtained from the local seismic response analysis.

### 6.3 Ground motion amplification factors

Finally, the amplification factors computed from the local seismic response analysis are reported in Table 4

Table 4. Amplification factor for the Cona area, computed from the local seismic response analysis.

codice	FA <sub>0.1-0.5</sub>	FA <sub>0.5-1.5</sub>	FH <sub>0.1-0.5</sub>	FH <sub>0.5-1.5</sub>
000046xa_038008	1.640	1.739	2.438	1.626
000126xa_038008	1.635	1.826	2.667	1.716
000354xa_038008	1.456	1.753	2.564	1.610
media	1.577	1.772	2.556	1.651

where:

$$FA_{0.1-0.5} = \frac{SI_{0.1-0.5}(PSV_{output})}{SI_{0.1-0.5}(PSV_{input})}$$

$$FA_{0.5-1.5} = \frac{SI_{0.5-1.5}(PSV_{output})}{SI_{0.5-1.5}(PSV_{input})}$$

$$FH_{0.1-0.5} = \frac{SI_{0.1-0.5}(PSA_{output})}{SI_{0.1-0.5}(PSA_{input})}$$

$$FH_{0.5-1.5} = \frac{SI_{0.5-1.5}(PSA_{output})}{SI_{0.5-1.5}(PSA_{input})}$$



## Bibliography

Albarelo, D. Lunedei, E. and Paolucci, E. (2013). Misure di sismica passiva nell'area comunale di Ferrara.

Albarelo D. and Castellaro S., 2011. Tecniche sismiche passive. *Ingegneria Sismica*, Anno XXVII, 2, (Suppl.), 32-63.

Bartolomei G., Bondesan M., Dal Cin R., Mase' G. and Vuillermin F., 1975 - Studio geologico coordinato per la pianificazione territoriale del Comune di Ferrara. *Memorie della Società Geologica Italiana*, vol. 14, pp.165-205.

Bondesan, M. and Gragini, A. (2003). Carta Geomorfologica del Comune di Ferrara. Conferenza delle Regioni e delle Province autonome, Commissione protezione civile, Sottocommissione 8, Attuazione della normativa sismica; Presidenza del Consiglio dei Ministri, Dipartimento della protezione civile (2009). *Indirizzi e Criteri per la Microzonazione sismica ICMS*.

D.G.R. 2007. *Indirizzi per gli studi di microzonazione sismica in Emilia-Romagna per la pianificazione territoriale e urbanistica*.

Gargini, A. and Bondesan, M. (2003). Supporto tecnico geologico-idrogeologico alla procedura di valutazione e sostenibilità ambientale per il nuovo piano regolatore del comune di Ferrara zona via Bologna – direttrice per Cona, Ferrara Nord-Pontelagoscuro.

Martelli L., P. Severi, G. Biavati, S. Rosselli, R. Camassi, E. Ercolani, A. Marcellini, A. Tinto, D. Gerosa, D. Albarelo, F. Guerrini, E. Lunedei, D. Pileggi, F. Pergalani, M. Compagnoni, V. Fioravante, D. Giretti, (2013). Analysis of the local seismic hazard for the stability tests of the main bank of the Po river (Northern Italy). *Boll. Geofis. Teor. Appl.*

Meletti, C. and Valenise, G. (2004). *Zonazione sismogenetica ZS) – App.2 al Rapporto Conclusivo*. 10

Norme Tecniche per le Costruzioni (2008).

Provincia di Ferrara (1982). Carta della falda freatica.

Provincia di Ferrara, Regione Emilia-Romagna, Università degli Studi di Ferrara. Risorse Idriche sotterranee della Provincia di Ferrara.

Rovida, R. Camassi, P. Gasperini and M. Stucchi, 2011. CPTI11, la versione 2011 del Catalogo Parametrico dei Terremoti Italiani. Milano, Bologna, <http://emidius.mi.ingv.it/CPTI> Regione Emilia Romagna, Eni-Agip, (1998). Riserve idriche sotterranee della Regione Emilia-Romagna. A cura di G. Di Dio. S.EL.CA. (Firenze)

Regione Emilia-Romagna (1999). Carta Geologica di Pianura Robertson, P.K., 1990. Soil Classification using the CPT. Canadian Geotechnical Journal. 27(1),151-158.

Robertson, P.K. and Wride, C.E., 1998. Cyclic Liquefaction and its Evaluation based on the CPT Canadian Geotechnical Journal, 1998, Vol. 35.

Robertson, P.K. (2004). Evaluating Soil Liquefaction and Post-earthquake deformations using the CPT.

P.K. Robertson and K.L. Cabal (2010). Estimating soil unit weight from CPT.

Seed, H. B., and Idriss, I. M. 1971. Simplified procedure for evaluation soil liquefaction potential. Journal of the Soil Mechanics and Foundations Division, ASCE, 97(SM9): 1249-1273.

Seed, H. B., and Idriss, I. M. 1982. Ground motions and soil liquefaction during earthquakes. Earthquake Engineering Research Institute, p.134.